# **Stress Analysis of a Boom of Pick-n-Carry Mobile Crane**

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# ABSTRACT

It is very difficult to build telescopic booms for mobile cranes that are both light and strong. Many techniques exist to retract and lengthen crane booms. While retraction of the crane's boom enhances performance, extending the boom limits its capacity. As the distance increases, it gets more difficult to communicate. The crane's ability to lift weight reduces as it gets farther away from the load. In the past, crane booms couldn't reach new heights, but now they can, making them more adaptable than ever. When the crane's boom is in the maximum angular position possible Crane boom and component stress analysis is used in this study. According to SAE J1078 [2], computations are made by hand. Maximizing the weight and strength of the crane boom will enable it to be more efficient. In this study, crane boom parts are compared.

# Keywords: Boom, Strength

# Introduction

Because of their mobility, these cranes are widely used in the construction business. Mobile cranes are able to lift and move enormous quantities of weight. loads. In addition, these cranes may be transported on public roads. Mobile cranes are not complete without their booms. You can't argue with the facts here. Booms that can be extended by at least two parts Stepping back so that you may soar even higher. Portable units are used for crane lifts. Strength and weight are the two most common causes of capacity issues. Things begin to change after a time. Crane accidents are most often caused by the following: The structural integrity of mobile cranes may be weakened [4]. [2] The Heavy-duty mobile cranes have a number of drawbacks. to meet the needs and desires of the customer long booms that may be used in a variety of circumstances too much weight on booms that are too small or light Boom's initial section's outermost rim Attempting to solve the issue gets more challenging. Having four pates in the boom makes it more difficult to do anything. sections. In the past, cranes had wooden boom sections. The most important component of the meal must be included to the next most strong and the biggest power Booster hose extensions The product's design is a major issue power, as well as the ability to lessen the symptoms of exhaustion Amount of weight carried by the boom of a mobile crane. As a second step, the gathering of data is necessary.

# **Purposes & Objectives**

A manual calculation's primary goal is to establish a value. how a person's abilities are evaluated. extensions for mobile cranes that can be extended at will Training in Analytical Reasoning 'An effective boom is one that has the potential to solve the situation at hand.' Some situations of interaction equations may have a result less than or equal to one. The numerous aspects of this issue have been taken into consideration: The panels are buckling and twisting in both directions due to torsional stress and bending. It's crucial to calculate the compressive stresses [2]. This

inquiry has found that. SAE is used to manually calculate the results. AISC and J1078 are the most widely used standards in the industry.

# Methods and approaches

A boom is a must for any lifting operation to be effective. An very unusual occurrence. The crane's boom must be inspected for safety. Versatile in its use. In this article, an example is provided. The crane boom is subjected to a stress test. Breaking breaking a section into smaller sections. Jib is used for Hydra crane's 44-foot boom, which is 44 feet long. You might also look at the boom's lifting capacity, which is 12 tonnes. capacity. As part of the boom's design, the weights, etc. The object's dimensions and cross-sectional shape are shown in 3D computer graphics elements like hydraulic cylinders and boom sections. The Mathematical calculations are required in order to carry out a boom-stress study. The following are examples of crane boom operating conditions: The boom has a 0° curve when completely extended. This is the point at which the boom reaches its maximum length a 55-degree incline is feasible. At a zero-degree angle, the boom has been fully retracted. The boom extends to a length of four feet when completely retracted. A 55 percent angle of view is achievable.

There are more than four possible scenarios in which forces and moments may be measured. This study examines some of the effects of stress. That concludes our look at the aforementioned four scenarios. The crane's retractable boom design The following images illustrate the user's current location and their stretched state. a minimum of two and a maximum of four distinct hypotheses There is minimal wind pressure on the head. Because of the side load, a lot of torque is generated on the head itself. The winch rope requires a 3 degree fleet angle.

# Negligible

The pressure of the wind is the same on each side. A reply is the focus of this section. Friction forces arise in axial strains in materials. Only a few reactivity points link the various components of the system. To other stressors and the weight it imposes. The axial loads are carried by the cylinders. How stress analysis calculations operate is explained here:

# Gather data as a starting point.

Included in this paper are details on boom height, operating distance, boom tilt, and rated load capacity. The first step is to figure out how the Boom is set up.b) The algorithm yields a shear diagram and a moment diagram.Secondly, the equations for the forces and moments have been deduced and studied. Build a crane boom and identify its components.

# Analysis

The material's characteristics are well understood. Secondly, we have established the section's attributes. Consider the song's compressive and sectional characteristics in this period. Real and permissible quantities of stress may be calculated. Interaction equations and solutions. Stresses caused by compression. Web shear stress is determined in this step. In step 5, use tensile forces.

# **Information Gathering**

The Solidworks boom design is used to gather the data. Section characteristics and boom distances are inferred from the layout itself. each and every one of the characteristics necessary for based on current data, stress analysis calculations may be made. The boom's construction design.

W1 = Weight of Fly Jib =80.198112kg = 196.7154 lb W2 = Weight of  $2^{nd}$  Extension = 264.7109 kg = 583.7872 lb W3 = Weight of  $1^{st}$  Extension = 401.292 kg = 884.9999 lb W4 = Weight of Mother Boom = 711.29kg = 1568.662 lb W5 = Weight of Extension Cylinder = 147 kg = 324.1903 lb W6 = Weight of Lug 1 = 43 kg = 94.83118 lb W7 = Weight of Lug 2 = 30.5 kg = 67.26398 lb W8 = Weight of Lug 3 = 25.6 kg = 56.45763 lb

W9 = Weight of Hook Block = 149 kg = 328.6011 lb

Table 1: Boom distance	es
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Boom Distances	Boom (Fully Extended) Boom length=13093mm	Boom (Fully Retracted) Boom length = 7801mm
Fly Jib		
Load pt. to 2nd ext. end pt. hor. (L1)	389.45mm(15.33268 in)	389.45 mm(15.33267in)
Load pt. to boom center line, ver. (L2)	103.35mm(4.068898 in)	103.35 mm(4.068897in)
Pulley center to 2nd ext. end pt., hor. (L3)	169.45mm (6.67126 in)	169.45 mm(6.671259 in)
Pulley outer to 2nd ext. end pt. ver. (L4)	185.65mm(7.309055 in)	185.65 mm(7.309055 in)
Pin point to flyjib head (L5)	191.38mm(7.534646 in)	191.38 mm(7.534645 in)
C.G. to flyib center line, hor. (L6)	1.12mm (0.044094 in)	1.12 mm (0.044094in)
Pin point to flyjib center line, ver. (L7)	117.5mm (4.625984 in)	117.5 mm (4.625984 in)
2nd Extension		, í
Bottom pad to end pt. (L8)	2803.6mm(110.378 in)	111.88 mm(4.404724in)
Bottom pad to top pad, hor. (L9)	971.4mm(38.24409 in)	3663.12mm(144.2173in)
Bottom pad to C.G. (L10)	854.43mm(33.63898 in)	3719.22mm(146.4259in)
1st Extension	, , , ,	, í
Bottom pad on 2nd ext. to bottom pad on 1st ext.	2925mm (115.1575 in)	3719.22mm(146.4259in)
Bottom pad to top pad, hor. (L12)	1953.6mm(76.91339 in)	4392 mm(172.913386in)
Bottom pad to C.G. (L13)	520.73mm(20.50118 in)	3394.22mm(133.6307in)
Bottom pad on 2nd ext. to top pad on 1st ext. (L14)	1792mm (70.55118 in)	4717 mm(185.708661in)
Bottom pad to ext. cyl. Point on 1st extension (L15)	985.05mm(38.7815 in)	3585.05mm(141.1437in)
Mother Boom	, , ,	
Top pad to lift cyl. (L16)	3377mm(132.9528 in)	777 mm(30.5905512 in)
Bottom pad to lift cyl. (L17)	5169mm (203.5039 in)	5169 mm(203.50397 in)
Lift cyl. to boom pivot pt. (L18)	1806mm (71.10236 in)	1806.00mm(71.10236in)
Boom pivot pt. to C.G. (L19)	3167.72mm(124.713in)	3167.72mm(124.7133in)
Boom pivot pt. to boom center line (L20)	457.65mm(18.01772 in)	457.65mm(18.017716in)
Boom pivot pt. to extension cyl. pt. (L21)	389.9mm(15.35039in)	389.9 mm(15.350393in)
Boom pivot pt. to lift cyl. Pt. on chassis, hor. (L22)	3019.98mm (118.8969)	3019.98mm(118.8969in)
Boom pivot pt. to lift cyl. Pt. on boom, ver. (L23)	386.65mm(15.22244 in)	386.65mm(15.22244in)
Boom pivot pt. to lift cyl. Pt. on chassis, ver. (L24)	1572.5mm(61.90945in)	1572.5mm(61.90945in)
Boom pivot pt. to lug1, hor. (L25)	4505mm (177.3622 in)	4505 mm(177.3622in)
Boom pivot pt. to lug2, hor. (L26)	4805mm (189.1732 in)	4805.00mm(189.1733in)
Boom pivot pt. to lug3, hor. (L27)	5205mm(204.9213 in)	5205.00mm(204.9212in)
Boom pivot pt. to lug1, ver. (L28)	156.65mm(6.167323 in)	156.65 mm(6.167322in)
Boom pivot pt. to lug2, ver. (L29)	156.65mm(6.167323 in)	156.65 mm(6.16732in)
Boom pivot pt. to lug3, ver. (L30)	156.65mm(6.167323 in)	156.65mm(6.167322in)
Breadth of Mother Boom (L31)	325mm(12,79528)	325mm (12,79528in)

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Extension Cylinder Data	Lift Cylinder Data
Bore = 100mm	Bore = 125mm
Stroke = 2100mm	Stroke = 1600mm
Closed Center Length =	Closed Center Length =
2500mm	1980mm
Width = 147 Kg	Number of Cylinders $= 2$

Table 2: Cylinder data

#### 2.5.2 Material properties of crane boom

The material of boom of mobile crane is Mild steel having IS: 2062 grade having Ultimate tensile strength = 410Mpa, Yield strength = 250N/mm<sup>2</sup> = 36.2594344325 ksi, Poisson's ratio = 0.29,

Mass density = 7.85kg/m<sup>2</sup>

Boom Sections	Mother Boom	1 <sup>st</sup> Extension	2 <sup>nd</sup> Extension	Fly Jib	
B(mm)	325	275	185	90	
H(mm)	450	380	298	231	
TTop(Tt)	8	8	8	4	
TBottom(Tb)	10	8	8	4	
TSide(Ts)	8	8	8	4	
Length (Ls)	7100	5000	3900	4080	
Ix(mm <sup>4</sup> )	39196.337	21655.059	9215.0811	1666.9225	
Zx(mm <sup>3</sup> )	1742.06	1139.74	618.462	144.322	
Iy(mm <sup>4</sup> )	22517.404	13155.701	4380.5343	378.69947	
Zy(mm <sup>3</sup> )	1385.7	956.78	473.57	84.155	
Area(mm <sup>2</sup> )	127.62	102.24	74.72	25.04	
Volume(mm <sup>3</sup> )	90610.2	51120	29140.8	10216.32	

# Table 3: Section Properties of 44' Crane Boom

In four circumstances, an external load is imposed. With rubber tyre mounted cranes, rated loads cannot exceed 85 percent of tipping load at the given radius.All external loads are taken into account. According to the load chart of a mobile crane (Figure 2).

Table 4: Load lifted in four working conditions of crane

boom					
External	Case-1	Case-2	Case-3	Case-4	
load	Boom	Boom	Boom	Boom	
P (Kg)	3420	7623	2100	4230	

### 2.5.4 Forces and moments in Boom sections [3]

### a) Maximum load calculation

Where  $Pz = P1 \times SA$   $Px = Fll \times P1$   $M1 = (Py \times Li1) + (Pz \times Li2) - P \times Li4 \div N$   $M2 = Px \times Li1$  $T = Px \times Li2$ 

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Figure 1: Loading diagram - Boom Assembly [3]



Figure 2: Load moment diagram - Head section [3]

b) Forces and moment in flyjib [3] Moments  $Mx1 = (Py \times Li1) + (Pz \times Li2) - (P1 \div N)$  $\times$  Li4 + Py  $\times$  Li5 + w1  $\times$  CA  $\times$  Li6  $My1 = Px \times Li1 + Px \times Li5 + 0.5$  $\times$  gi  $\times$  d1  $\times$  (Li5  $\wedge$  2) Axial Force  $Rz1 = (P1 \div N) + Pz + (w1 \times SA)$  $Par1 = (P1 \div N) + Pz + (w1 \times SA)$  $Pal1 = w2 \times SA$ Vertical reactions  $Ry1 = PY + w1 \times CA$  $Rx1 = PX + gi \times d1 \times Li5$  $Vyr1 = Py + w1 \times CA$  $Vyl1 = Ry + w1 \times CA$  $Vxr1 = Px + gi \times d1 \times Li5$ Vxl1 = 0

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Figure 3: Load moment diagram - Flyjib [3]

c) Forces and moments in 2<sup>nd</sup> Extension [3] Moments

 $Mx2 = Mx1 + Ry1 \times Li8 +$  $(0.5 \times w2 \times CA \times ((Li8 \wedge 2) \div (Li8 + Li9)))$  $My2 = My1 + Rx1 \times Li8 +$  $(0.5 \times gi \times d2 \times (Li8 \wedge 2))$ Axial Force  $Rz2 = Rz1 + w2 \times SA$  $Par2 = Rz1 + ((w2 \times SA \times Li8) \div (Li8 + Li9))$ Vertical Reactions Ry3 =  $(Mx2 \div Li9) - ((0.5 \times w2 \times CA \times Li9))$ + (Li9 + Li8)) $Ry2 = Ry1 + Ry3 + w2 \times CA$  $Rx3 = My2 \div Li9$  $Rx2 = Rx1 + Rx3 + gi \times d2 \times Li8$  $Vyr2 = Ry1 + ((w2 \times CA \times Li8) \div (Li8 + Li9))$  $Vyl2 = Ry3 + ((w2 \times CA \times Li9) \div (Li9 + Li8))$  $Vxr2 = Rx1 + gi \times d2 \times Li8$ Vxl2 = Rx3



Figure 4: Load moment diagram 2<sup>nd</sup> Extension [3]

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d) Forces and moments in 1<sup>st</sup> extension [3]



Figure 5: Load moment diagram 1st Extension [3]

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Moments
Mx3 = Ry2 \times Li11 - Ry3 \times Li14 - w3 \times CA \times Li12 - w6 \times CA \times Li15
My3 = Rx2 \times Li11 - Rx3 \times Li14 + (0.5 \times gi \times d3 \times (Li11 \wedge 2))
Rz3 = Rz2 + w3 \times SA
Par3 = Rz2 + ((w3 \times SA \times Li11) + (Li11 + Li12))
Pal3 = ((w3 \times SA \times Li12) \div (Li11 + Li12))
Vertical reactions
Ry5 = (Mx3 \div Li12) - ((0.5 \times w3 \times CA \times Li12) \div (Li11 + Li12))
Ry4 = Ry2 - Ry3 + Ry5 + w3 \times CA + w6 \times CA
Rx5 = (My3 \div Li12)
Rx4 = Rx2 - Rx3 + Rx5 + gi \times d3 \times Li11
Vyr3 = Ry2 - Ry3 + ((w3 \times CA \times Li11) \div (Li11 + Li12)) + w6 \times CA
Vyl3 = Ry5 + ((w3 \times CA \times Li12) \div (Li11 + Li12))
Vxr3 = Rx2 - Rx3 + gi \times d3 \times Li11
Vxl3 = Rx5
      e) Forces and moments in mother boom [3]
      Moments
          Mx4 = (Ry4 \times Li16) - (Ry5 \times Li17) + ((0.5 \times w4 \times CA \times (Li16 \land 2)) \div (Li16 +
              Li17) + w7 × Li25 + w8 × Li26 + w9 × Li27 + w5 × Li27 + w5 × Li21
      My4 = (Rx4 \times Li16) - (Rx5 \times Li17) + (0.5 \times gi \times d4 \times (Li16 \wedge 2))
      Axial load on cylinder
      Rz4 = Rz3 + (w4 + w5 + w7 + w8 + w9) \times SA
      Axial load on section
      Par4 = (w4 \times Li16) \div (Li16 + Li18)
      Pal4 = Par3
Derrick cylinder reaction
 Rd = (Ry4 \times (Li16 + Li18) - Ry5 \times (Li17 + Li18) + w4 \times (Li19 \times CA - Li20 \times SA) +
      (w5 \times Li21 \times CA - Rz4 \times Li20) \div ((Li18 - ((Li20 - (d4 \div 2)) \div OT)) \times CT)
Pivot Pin loading
Rx6 = Rx4 - Rx5 + gi \times d4 \times (Li16 + Li18)
    Rzr6 = (Rd \times ST \div 2) + (Rx4 \times (Li16 + Li18) \div Li31) - (Rx5 \times (Li17 + Li18) \div
 Li31) - (Rz4 \div 2) + (w4 \times SA \div 2) + ((0.5 \times gi \times d4 \times ((Li16 + Li18) \land 2) \div li31) +
                                 ((w7 + w8 + w9) \times SA \div 2)
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$$\begin{split} & \text{RZL6} = (\text{Rd} \times \text{ST} + 2) - (\text{Rx4} \times (\text{Li16} + \text{Li18}) \div \text{Li31}) + (\text{Rx5} \times (\text{Li17} + \text{Li18}) \div \\ & \text{Li31}) - (\text{Rz4} + 2) + (\text{w4} \times \text{SA} \div 2) - ((0.5 \times \text{gi} \times \text{d4} \times ((\text{Li16} + \text{Li18}) \land 2) \div \text{Li31}) + \\ & ((\text{w7} + \text{w8} + \text{w9}) \times \text{SA} \div 2) \\ & \text{Ryr6} = 0.5 \times (\text{Rd} \times \text{CT} + \text{Ry5} - \text{Ry4} - (\text{w4} + \text{w5} + \text{w7} + \text{w8} + \text{w9}) \times \text{CA}) - \\ & (\text{Px} \times (\text{Li2} - \text{Li20}) \div \text{Li31}) + (\text{gi} \times \text{d4} \times (\text{Li16} + \text{Li18}) \times \text{Li20} \div \text{Li31}) \\ & \text{Ryl6} = 0.5 \times (\text{Rd} \times \text{CT} + \text{Ry5} - \text{Ry4} - (\text{w4} + \text{w5} + \text{w7} + \text{w8} + \text{w9}) \times \text{CA}) + \\ & (\text{Px} \times (\text{Li2} - \text{Li20}) \div \text{Li31}) + (\text{gi} \times \text{d4} \times (\text{Li16} + \text{Li18}) \times \text{Li20} \div \text{Li31}) \\ & \text{Ryl6} = 0.5 \times (\text{Rd} \times \text{CT} + \text{Ry5} - \text{Ry4} - (\text{w4} + \text{w5} + \text{w7} + \text{w8} + \text{w9}) \times \text{CA}) + \\ & (\text{Px} \times (\text{Li2} - \text{Li20}) \div \text{Li31}) - ((\text{gi} \times \text{d4} \times (\text{Li16} + \text{Li18}) \times \text{Li20}) \div \text{Li31}) \\ & \text{Vertical shear force} \\ & \text{Vyr4} = \text{Ry4} - \text{Ry5} + ((\text{w4} \times \text{CA} \times \text{Li16}) \div (\text{Li16} + \text{Li18})) \\ & \text{Vyl4} = \text{Ry4} - \text{Ry5} + ((\text{w4} \times \text{CA} \times \text{Li16}) \div (\text{Li16} + \text{Li18})) \\ & \text{Vyl4} = \text{Ry6} + \text{Ryl6} + ((\text{w4} \times \text{CA} \times \text{Li16}) \div (\text{Li16} + \text{Li18})) + (\text{w5} + \text{w7} + \text{w8} + \text{w9}) \times \\ & \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} \times \text{CA} + \text{CA} \times \text{CA} \times$$

CA

Lateral shear force

 $Vxr4 = Rx4 - Rx5 + gi \times d4 \times Li16$  $Vxl4 = Rx4 - gi \times d4 \times \times Li18$ Extension cylinder reaction

 $Recy = (w1 + w2 + w3 + w6 + P) \times SA$ 



Figure 6: Mother Boom [3]

f) Calculations for forces and moments

Table 5: Maximum load calculation in four working conditions of boom

Max. load	Case-1	Case-2	Case-3	Case-4
calculation	Boom retracted at a=0	Boom retracted at a=55	Boom extended at a=0	Boom extended at a=55
P1	7542.386912	16811.58346	4631.290209	9328.741707
Py	7542.386912	371.9858096	4631.290209	206.414794
Pz	0	-16807.46754	0	-9326.457782
Px	452.5432147	1008.695008	277.8774125	559.7245024
M1	107312.6607	-81256.63568	65893.73905	-45089.27836
M2	6938.699014	15465.99491	4260.604658	8582.075097
Т	1841.352017	4104,276733	1130.654747	2277.461706

	Table 6: Forces and Moments on Flyjib in four working conditions of boom					
Forces & Moments	Case-1	Case-2	Case-3	Case-4		
in Fly Jib	Boom retracted at $\alpha=0$	Boom retracted at a=55	Boom extended at a=0	Boom extended at a=5.		
Mx1	154106.0881	-100840.3028	94630.43061	-55956.17152		
Myl	10348.45181	23066.1544	6354.312523	12799.40092		
Rz1	2514.128971	-11380.4301	1543.763403	-6393.700926		
Par1	2514.128971	-11380.4301	1543.763403	-6393.700926		
Pal1	0	-583.64426	0	-583.64426		
Ry1	7719.253927	375.8993029	4828.00561	210.3282873		
Rx1	452.5432206	1008.695013	277.8774184	559.7245083		
Vyr1	7719.253927	375.8993029	4828.00561	210.3282873		
Vy11	7896.120941	379.8127962	5024.721012	214.2417806		
Vxr1	452.5432206	1008.695013	277.8774184	559.7245083		
Vx11	0	0	0	0		

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Forces & Mome	mts Case-1	Case-2	Case-3	Case-4
in 2 <sup>nd</sup> Extensio	on Boom retracted	at Boom retracted at	Boom extended at	Boom extended at
	a=0°	a=55°	a=0°	a=55°
Mx2	188145.379	-99183.72686	651463.7831	-46729.45479
My2	12341.77999	27509.17796	37025.85919	36960.10858
Rz2	2514.128971	-11964.07436	1543.763403	-6977.345186
Par2	2514.128971	-11397.72761	1543.763403	-6703.163914
Pal2	0	-566.3467448	0	-274.181272
Ry3	1021.353535	-694.0051988	16959.2515	-1224.907855
Ry2	9324.394649	-305.1885791	22371.04429	-1001.662251
Rx3	85.57765285	190.7480836	968.1457931	966.4265574
Rx2	538.1208778	1199.443101	1246.023322	1526.151109
Vyr2	7736.555678	376.2821345	5261.570045	217.1773762
Vyl2	1587.838971	-681.4707137	17109.47424	-1218.839627
Vxr2	452.543225	1008.695018	277.877529	559.7245515
Vx12	85.57765285	190.7480836	968.1457931	966.4265574
Ta	ble 8: Forces and Mo	ments on 1 <sup>st</sup> extension i	n four working conditio	ns of boom
Forces &	Const Deres	() )	(Inc. 2)	Creat Dame
Moments in	Case-1 Doom	Case-2 Boom	Case-5 Boom	Case-4 Doom
1 <sup>st</sup> Extension	retracted at a=0°	retracted at a=55°	extended at a=0°	extended at a=55°
Mx3	-236779.145	-17316.44824	1307951.838	28784.93906
My3	-9007.106321	152948.0703	75185.08548	86526.85779
Rz3	2514.128971	-12848.85755	1543.763403	-7862.128377
Par3	2514.128971	-12349.96065	1543.763403	-7841.970732
Pal2	0	-566.3467448	0	-274.181272
Ry5	-1781.362965	-105.6660519	16828.32043	8308.15707
Ry4	7501.509191	304.8310505	23219.94426	8553.083157
Rx5	-52.09027791	884.5357435	977.5292646	24974.79759
Rx4	-61.51303245	1044.541862	2235.05866	1166.037355

Table 7: Forces and Moments on 2nd extension in four working conditions of boom

Table 0: Forces and	Momente on	Mother ho	om in four	nocitions of hoon	•
Table 9: Forces and	i Moments on	womer bo	om in iour	DOSITIONS OF DOOD	1

6037.232056

17182.71221

277.8776762

977.5292646

244.4799542

8308.603203

559.7247415

24974.79759

399.4554291

-94.62437859

1008.695189

884.5357435

8458.84854

-957.3393497

452.5432413

-52.09027791

Vyr3 Vyl3

Vxr3

Vx13

Forces & Moments in Mother Boom	Case-1 Boom retracted at a=0°	Case-2 Boom retracted at a=55°	Case-3 Boom extended at a=0°	Case-4 Boom extended at a=55°
MX4	622711.8133	58484.59092	-201343.3013	-524573.7277
MY4	8718.859774	-148053.3942	98226.16802	-4927441.742
RZ4	2514.128971	-14864.93786	1543.763403	-10762.9921
PAR4	471.8739654	471.8739654	1022.066741	1598.691336
PAL4	2514.128971	-12349.96065	1543.763403	-7841.970732
Rđ	-20926.87593	7006.081268	-4308.992559	-5657.970635
Rx6	-9.422600613	160.0062728	1257.529705	-23808.75992
Rzró	-628.0130257	-4095.315699	13892.86666	-489031.9266
Rz16	-1886.115945	17268.28419	-15436.63006	497218.1659
Ryró	5041.759233	3054.470077	-1965.600278	-1335.467486
Ryl6	4055.075347	855.2041597	-2571.459408	-2498.579113
Vyr4	9754.746121	420.9381426	7413.690575	280.2999403
Vyl4	10016.62047	3930.026115	-2738.479954	-3788.761907
Vxr4	147.090947	-853.7935171	-1266.912751	-199.6107923
Vx14	-61.51314008	1044.541755	2235.058553	1166.037247
Recy	0	-9361.192813	0	-5969.02351

# 2.6 Equations used for stress analysis:

2.6.1 Calculation of section properties based on compressive stresses Btf = B ÷ Tt Btw = H ÷ Ts Bta = 184 ÷ Sqrt(Fost × Fyi)

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 $Fa = Par \div (As \times 1000)$  $Fbx = Mx \div (Zx \times 1000)$  $Fby = My \div (Zy \times 1000)$ Ff = Fa + FbxFw = Fa + FbyIf  $(Btf \leq Bta)$  and  $(Btw \leq Bta)$  Then the plates in compression are fully effective at yield [10].  $Btxr = 184 \div Sqrt(Abs(Ff))$  $Btyr = 184 \div Sqrt(Abs(Fw))$ If  $(BTF \leq BTXR)$  and  $(BTW \leq BTYR)$  Then the plates in compression are fully effective at actual stress [10].  $Btq = Ts \div Tb$ If Btq  $\leq$  95 ÷ Sqr (FYi) Then Qs = 1 Sigr =  $0.5 \times Fyi$  $Rx = Sqrt(Ix \div As)$ Ry = Sqrt(Iy + As) $Cc = Sqrt(((Pi) \land 2) \times E \div (Qs \times Qa \times (Fyi - Sigr)))$  $Klx = k \times Ls \div Rx$  $Kly = k \times Ls \div Ry$ If Klx < Kly then Kl = Kly If Kl > Cc then, Elastic range  $Faa = 12 \times (Pi \land 2) \times E \div (23 \times (Kl \land 2))$ If Kl < Cc Faa = Qs × Qa ×  $(1 - \text{Sigr} \times ((\text{Kl}) \land 2) \div$  $(Fyi \times (Cc \wedge 2)) \times Fyi \div (5 \div 3) + (3 \div 8)$  $\times$  (Kl ÷ Cc) – (1 ÷ 8)  $\times$  ((Kl ÷ Cc)  $\wedge$  3)) 2.6.2 Inelastic lateral Buckling If M1 > M2 then Mxmin = M1 And Mxmax = Mx Bm = B - Ts $Hm = H - ((Tt + Tb) \div 2)$  $J = 4 \times (Bm \land 2) \times (Hm \land 2) \div$  $((2 \times Hm \div Ts) + (B \div Tb) + (B \div Tt))$ 2.6.3 Inelastic Lateral buckling check  $Cb = 1.75 + 1.05 \times (Mxmin \div Mxmax)$  $+0.3 \times ((Mxmin \div Mxmax) \land 2)$ Where  $1 \le Cb \le 1.3$  $Kle = Sqrt(5.1 \times Kt \times Ls \times Zx \div Sqrt(J \times Iy))$ If Kle  $\leq$  (102000 ÷ FYi) than

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 $Fbxa = Fost \times Fyi$  $Fbya = Fost \times Fyi$ 

2.6.4 Solution to interaction equations for compressive stresses

 $\begin{array}{l} Xa = Abs(Fa \div Faa), Xb = Abs(Fbx \div Fbxa), \\ Xc = Abx(Fby \div Fbya) \\ Fex = 12 \times (Pi \wedge 2) \times E \div (23 \times (Klx \wedge 2)) \\ Fey = 12 \times (Pi \wedge 2) \times E \div (23 \times (Kly \wedge 2)) \\ If Xa \leq 0.15 \ than \ Xd = Xa + Xb + Xc \\ If Xd \leq 1 \ than \ the \ design \ will \ be \ safe \ against \ buckling \\ And \ if Xd > 1, \ Than \ Xd = (Fa \div (FOST \ x \ Fyi)) + Xb + Xc \\ And \\ Xd1 = Xa + Cmx \times Fbx \div ((1 - (Fa \div Fex))) \end{array}$ 

 $\times$  (Fbxa) + Cmy  $\times$  Fby  $\div$ 

$$((1 - (Fa \div Fey)) \times Fbya)$$

If Xd and Xd1 equal to or less than one than the design will be safe

2.6.5 Actual and allowable shear stresses in webs

$$\begin{split} Fs &= (Vyr \div (2 \times B \times Ts) + T \div \\ & (2 \times As \times Ts)) \div 1000 \\ If H \div T \leq 380 \div Sqrt (Fyi) \ than \ Fsa = 0.4 \ X \ Fyi \\ And \ if \ Fsa = 0.4 \leq Fyi \ than \ stiffeners \ are \ not \ required \\ If \ Abs \ (Fs) \leq Abs \ (Fsa) \ than \ the \ design \ will \ be \ safe \ against \ shear \end{split}$$

2.6.6 Tensile stresses Ft = (-Fa + Fbx + Fby)

Ft = (-Fa + Fbx + Fby)Fta = (Fost × Fyi) If Ft  $\leq$  Fta, than the design will be safe against tensile failure

2.7 Calculations for stress analysis

2.7.1 Case – 1 When the boom is fully retracted and at an angle =  $0^{\circ}$ 

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btf	22.5	23.125	34.375	40.625
Btw	57.75	37.25	47.5	56.25
Bta	37.52760125	37.52760125	37.52760125	37.52760125
Fa	0.647769747	0.217079155	0.158647833	0.023854741
Fbx	17.49799981	4.985189659	-3.404386091	5.857673294
Fby	2.015089483	0.427064602	-0.154267742	0.261896522
Ff	18.14576956	5.202268814	-3.245738257	5.881528035
Fw	2.662859231	0.644143757	0.004380091	0.285751263
Btxr	43.19466672	80.67174073	102.1318211	75.87046351
Btyr	112.7570535	229.259015	278.201253	344.2102065

Table 10: Calculation of section properties based on Compressive stresses

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Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btq	1	1	1	0.8
Qs	1	1	1	1
Qa	1	1	1	1
Sigr	18.21210326	18.21210326	18.21210326	18.21210326
Rx	3.212231386	4.372173945	5.729818869	6.899691211
Ry	1.531074066	3.014471493	4.465939585	5.22957065
Cc	126.4389349	126.4389349	126.4389349	126.4389349
Klx	100.0114263	81.27634792	68.71086092	81.02610697
Kly	209.8264543	117.8827969	88.15631737	106.9026801
K1	209.8264543	117.8827969	88.15631737	106.9026801
Faa	3.450282756	10.75387107	14.62058838	12.26578541

Table 11: Calculation for determination of allowable stresses

#### Table 12: Inelastic lateral buckling check

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom	
Mxmax	107312.6607	107312.6607	-236779.145	622711.8133	
Mxmin	154106.0881	188145.379	107312.6607	107312.6607	
Bm	3.385826772	6.968503937	10.51181102	12.48031496	
Hm	8.937007874	11.41732283	14.64566929	17.36220472	
J	23.10704846	213.2229929	586.1236399	14.4505726	
Cb	1.3	1.3	1.3	1.939857148	
Kle	22.59101563	15.3019954	12.89915202	1024.190401	
Fbxa	24.0399763	24.0399763	24.0399763	24.0399763	
Fbya	24.0399763	24.0399763	24.0399763	24.0399763	

Table 13: Solution to interaction equations for the Compressive Stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Xa	0.18774396	0.020186141	0.010850988	0.00194482
Xb	0.727870926	0.207370823	0.141613538	0.243663855
Xc	0.08382244	0.017764768	0.006417134	0.010894209
Fex	15.18713766	22.9957093	32.17543233	23.13796849
Fey	3.450282756	10.93136281	19.54646796	1.358038263
Xd	0.999437326	0.245321731	0.15888166	0.256502883

Table 14: Calculation of Actual and Allowable shear stress in the webs

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Fs	8.423187941	1.938650714	1.424902372	13.29223421
H/Ts	57.75	37.25	47.5	56.25
Fsa	14.56968261	14.56968261	14.56968261	14.56968261

#### Table 15: Calculation of tensile stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Ft	18.86531955	5.195175106	-3.717301666	6.095715074
Fta	24.0399763	24.0399763	24.0399763	24.0399763

2.7.2 Case – 2 When the boom is fully retracted and at an angle =  $55^{\circ}$ 

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Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btf	22.5	23.125	34.375	40.625
Btw	57.75	37.25	47.5	56.25
Bta	37.52760125	37.52760125	37.52760125	37.52760125
Fa	-2.93218781	-0.984121781	-0.77931344	0.023854741
Fbx	-11.4499279	-2.628019312	-0.248974104	0.550147948
Fby	4.491528395	0.951904517	2.619593083	-4.447217864
Ff	-14.3821157	-3.612141093	-1.028287544	0.574002689
Fw	1.559340583	-0.032217263	1.840279643	-4.423363123
Btxr	48.51839586	96.81339948	181.4514886	242.8627227
Btyr	147.3490473	1025.117154	135.6362931	87.48659752

Table 16: Calculation of section properties based on Compressive stresses

Table 17: Calculation for determination of allowable stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btq	1	1	1	0.8
Qs	1	1	1	1
Qa	1	1	1	1
Sigr	18.21210326	18.21210326	18.21210326	18.21210326
Rx	3.212231386	4.372173945	5.729818869	6.899691211
Ry	1.531074066	3.014471493	4.465939585	5.22957065
Cc	126.4389349	126.4389349	126.4389349	126.4389349
Kix	100.0114263	81.27634792	68.71086092	81.02610697
Kly	209.8264543	117.8827969	88.15631737	106.9026801
Kİ	209.8264543	117.8827969	88.15631737	106.9026801
Faa	3.450282756	10.75387107	14.62058838	12.26578541
Гdd	3.430282730	10.7558/107	14.02008808	12.20378341

# Table 18: Inelastic lateral buckling check

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Mxmax	-100840.303	-99183.72686	-17316.44824	58484.59092
Mxmin	-81256.6357	-81256.63568	-81256.63568	-81256.63568
Bm	3.385826772	6.968503937	10.51181102	12.48031496
Hm	8.937007874	11.41732283	14.64566929	17.36220472
J	23.10704846	213.2229929	586.1236399	1024.190401
Сь	1.3	1.3	1.3	1
Kle	22.59101563	15.3019954	12.89915202	16.47618776
Fbxa	24.0399763	24.0399763	24.0399763	24.0399763
Fbya	24.0399763	24.0399763	24.0399763	24.0399763

Table 19: Solution to interaction equations for the Compressive Stresses.

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Xa	0.84983986	0.091513258	0.039869783	0.00194482
Xb	0.476286988	0.109318715	0.01035667	0.022884713
Xc	0.186835808	0.039596733	0.108968206	0.184992606
Fex	15.18713766	22.9957093	32.17543233	23.13796849
Fey	3.450282756	10.93136281	19.54646796	13.29223421
Xd	Xd = 0.663123	0.240428706	0.159194659	0.209822138
	Xd1 =0.669321			

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Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Fs	3.694306965	0.644589912	0.469717905	0.381606865
H/Ts	57.75	37.25	47.5	56.25
Fsa	14.56968261	14.56968261	14.56968261	14.56968261

Table 20: Calculation of Actual and Allowable shear stress in the webs

# Table 21: Calculation of tensile stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Ft	-4.0262117	-0.691993014	3.149932419	-3.920924657
Fta	24.0399763	24.0399763	24.0399763	24.0399763

# 2.7.3 Case – 3 When the boom is fully extended and at an angle = $0^{\circ}$

Table 22: Calculation of section properties based on Compressive stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btf	22.5	23.125	34.375	40.625
Btw	57.75	37.25	47.5	56.25
Bta	37.52760125	37.52760125	37.52760125	37.61277208
Fa	0.397753353	0.133294218	0.097415336	0.051668749
Fbx	10.744827	17.26149498	18.80559643	-1.86861933
Fby	1.237335649	1.281211771	1.287720267	2.950511007
Ff	11.14258036	17.3947892	18.90301177	-1.816950581
Fw	1.635089003	1.41450599	1.385135603	3.002179756
Btxr	55.12199605	44.11723016	42.32064915	136.504279
Btyr	143.8954707	154.7089461	156.3405649	106.1938771

#### Table 23: Calculation for determination of allowable stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btq	1	1	1	0.8
Qs	1	1	1	1
Qa	1	1	1	1
Sigr	18.21210326	18.21210326	18.21210326	18.12971722
Rx	3.212231386	4.372173945	5.729818869	6.899691211
Ry	1.531074066	3.014471493	4.465939585	5.22957065
Cc	126.4389349	126.4389349	126.4389349	126.4389349
Klx	100.0114263	81.27634792	68.71086092	81.02610697
Kly	209.8264543	117.8827969	88.15631737	106.9026801
K1	209.8264543	117.8827969	88.15631737	106.9026801
Faa	3.450282756	12.92512958	14.62058838	12.2423439

#### Table 24: Inelastic lateral buckling check

			· · · · · ·	
Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Mxmax	94630.43061	651463.7831	1307951.838	-198647.3593
Mxmin	65893.73905	65893.73905	65893.73905	65893.73905
Bm	3.385826772	6.968503937	10.51181102	12.48031496
Hm	8.937007874	11.41732283	14.64566929	17.36220472
J	23.10704846	213.2229929	586.1236399	1024.190401
Сь	1.3	1.3	1.3	1.3
Kle	22.59101563	15.3019954	12.89915202	14.4505726
Fbxa	24.0399763	24.0399763	24.0399763	24.0399763
Fbya	24.0399763	24.0399763	24.0399763	24.0399763

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Parameters	Flyjib	2 <sup>nd</sup> extension	3 <sup>rd</sup> extension	Mother boom
Xa	0.049756429	0.010312795	0.006662888	0.004220495
Xb	0.446956639	0.718032945	0.782263518	0.073594439
Xc	0.05146992	0.053295051	0.053565788	0.123291256
Fex	15.18713766	30.79273489	32.17543233	23.13796849
Fey	3.450282756	14.63779841	19.54646796	13.29223421
Xd	0.548182987	0.781640792	0.842492193	0.20110619

Table 25: Solution to interaction equations for the Compressive Stresses

Table 26: Calculation of Actual and Allowable shear stress in the webs

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Fs	5.251086953	1.301788484	0.998486465	0.998248754
H/Ts	57.75	37.25	47.5	56.25
Fsa	14.56968261	14.56968261	14.56968261	14.56968261

#### Table 27: Calculation of tensile stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom	
Ft	11.5844093	18.40941253	19.99590136	1.030222927	
Fta	24.0399763	24.0399763	24.0399763	24.0399763	

# 2.7.4 Case – 4 When the boom is fully extended and at an angle = $55^{\circ}$

In this case the side thickness is taken as 16mm and total breadth of mother boom is taken 341mm. Because the strength required for mother boom is high when the boom has to work at 55 degree angle and boom is in extended position.

Table 28:	Calculation	of section	properties	based on
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Compressive stresses					
Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom	
Btf	22.5	23.125	34.375	42.625	
Btw	57.75	37.25	47.5	28.125	
Bta	37.52760125	37.52760125	37.52760125	37.52760125	
Fa	-1.64734828	-	-	0.051668756	
Fbx	- <b>6</b> .35355222	-	0.410112361	-3.76680393	
Fby	2.492347518	1.278936593	1.481974619	-	
Ff	-8.0009005	-	-	-	
Fw	0.84499924	0.703790157	0.992129202	-	
Btxr	65.05016286	136.6211111	651.6263213	95.4619965	
Btyr	200.1657019	219.329086	184.7284162	32.34760603	

Table 29: Calculation for determination of allowable etraceac

		Sucsses		
Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Btq	1	1	1	1.6
Qs	1	1	1	1
Qa	1	1	1	1
Sigr	18.21210326	18.21210326	18.21210326	18.21210326
Rx	3.212231386	4.372173945	5.729818869	6.314209581
Ry	1.531074066	3.014471493	4.465939585	5.743301605
Cc	126.4389349	126.4389349	126.4389349	126.4389349
Klx	100.0114263	70.23659581	68.71086092	88.53920842
K1y	209.8264543	101.8707972	88.15631737	97.34037259
K1	209.8264543	101.8707972	88.15631737	97.34037259
Faa	3.450282756	12.92512958	14.62058838	13.50143141

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	Table 50. Inclastic fateral oucking check					
Parameter	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom		
Mxmax	-55956.1715	-46749.53354	28523.80769	-524573.7277		
Mxmin	-45089.2784	-45089.27836	-45089.27836	-45089.27836		
Bm	-55956.1715	6.968503937	10.51181102	12.79527559		
Hm	3.385826772	11.41732283	14.64566929	17.36220472		
J	23.10704846	213.2229929	586.1236399	1497.231279		
Cb	1.3	1.3	1	1.3		
Kle	22.59101563	14.22485027	14.70729614	12.83437332		
Fbxa	24.0399763	24.0399763	24.0399763	24.0399763		
Fbya	24.0399763	24.0399763	24.0399763	24.0399763		

Table 30: Inelastic lateral buckling check

#### Table 31: Solution to interaction equations for the

	Compressive Stresses					
Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom		
Xa	0.477453123	0.044498311	0.03350381	0.003826909		
Xb	0.236758298	0.051526587	0.017059599	-0.15668917		
Xc	0.103675124	0.05320041	0.06164626	-1.348063316		
Fex	15.18713766	30.79273489	32.17543233	19.37777468		
Fey	3.450282756	14.63779841	19.54646796	16.03205385		
Xd	0.817886545	0.149225308	0.112209669	-1.500925576		

Table 32: Calculation of Actual and Allowable shear stress in the webs

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Fs	2.051452949	0.359306127	0.2637354	0.074997356
H/Ts	57.75	37.25	47.5	28.125
Fsa	14.56968261	14.56968261	14.56968261	14.56968261

#### Table 33: Calculation of tensile stresses

Parameters	Flyjib	2 <sup>nd</sup> extension	1 <sup>st</sup> extension	Mother boom
Ft	-2.21385643	0.6153851	2.381932396	-36.22588286
Fta	24.0399763	24.0399763	24.0399763	24.0399763

# CONCLUSION

In order to ensure the safety of the boom design under high loading circumstances, four distinct human computations are performed.Conditions at the workplace As it retracts, the boom drops to its lowest point.Zero-degree boom angle reduces the strain on the system.Boomed at a 55-degree angle for maximum effect.However, at times, the boom is expanded and used. The manual method does not disclose as much information as the automatic one does. According to the calculations, the boom will not break under stress. If the boom angle surpasses 30 degrees, it is not safe to operate at a zero-degree boom angle. 55°. Plates that are under or overstressed are found in compression. Responsive, but unable to provide results when put under real-world stress. When the boom is at a 55-degree angle, the stiffeners kick in. It is crucial for a child's stamina to be nurtured by their mother.When the manual control is engaged, the boom may be positioned at 55 degrees. The findings are more accurate because of stiffeners in the calculations satisfactory.

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